# **Auckland Harbour Bridge Alliance**

Principal Structural Inspection Concrete Piers Above Water (Boat)





## **Auckland Harbour Bridge Alliance**

### **Principal Structural Inspection**

## Concrete Piers Above Water (Boat) - 2020/2021

Inspection Details		
Inspector	Date of Inspection	
s 9(2)(a)	22/04/21	
	22/04/21	

<b>Principal Struct</b>	Principal Structural Inspection				2
Concrete Piers	Above \	Water (Boat) – 2020	/2021		1981
				OFA	
Inspection Details				2P	
Inspector	Date of	Inspection		101	
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	22/04/2	1	2		
			CO,		
Quality Assurance	Statement	17	71		
Report Prepared by:		s 9(2)(a)			
Report Reviewed by:		s 9(2)(a) (05/05/2021)	, s 9(2)(a)	(27/08/2021)	
Approved for issue by	<i>r</i> :	s 9(2)(a) (1/9/2021)			
Issued to:		s 9(2)(a) (02/09/2021)			

Revisio	Revision Schedule				
Rev. No	Date	Description	Prepared by	Reviewed by	Approved by
/5	)				
5					

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### 1 Scope

A Concrete Piers Above Water (Boat) Inspection was undertaken in April 2021 as part of the 2020-2021 Structural Inspection Programme. The scope of the inspection included the areas of the piers that had not already been inspected as part of the structural inspection 6 year programme. Table 1 outlines the scope of the inspections.

Report	Scope
Pier Bracket Interface Principal Inspection 2018- 19	Piers 1, 2 and 3. All concrete added for the installation of the pier brackets. Including concrete joins on the north and south faces, concrete corbel area underneath the pier bracket via abseil inspection.
Concrete Piers Principal (piers 4-6) 2019-2020	All areas of the concrete piers 4, 5 and 6. Including outside and inside faces via abseil inspection. Tidal zone was not included.
Concrete Piers Above Water (Boat) Principal Inspection 2020-2021	Tidal zone of all piers 1-6. North and south faces of piers 1, 2 and 3 (not included in the 2018-2019 inspection).

The inspection was carried out between 7.30 and 11:00 am on the 22nd of April 2021 at low tide where the water level was between 1.0-1.5m above lowest astronomical tide (LAT). This allowed for inspection of parts of the tidal zone of the piers.

Figure 1 shows the areas where close up visual inspection and hammer tapping was conducted.





The inspection primarily focussed on locating any signs of concrete deterioration and reinforcement corrosion such as spalling and/or rust staining in the chloride affected areas of the tidal zones. Visual inspection and hammer tapping of these areas was undertaken.

Large areas of oyster and muscle shellfish growth prevented thorough inspection of the bottom half of the tidal zone on the piers from approximately 1.0m below MHWS (Mean High Water Springs) to the water level. However small areas of growth were removed and the concrete underneath was inspected.

Appendix B shows a map of the defects found during this inspection. Each pier has a numbering system based on each recess in the concrete. Numbered from top to bottom.

### 2 Condition Rating

The condition of the areas covered in this inspection as per the description above can be described as good. The tidal zone area is in good condition. There was one crack found in the tidal zone on the north face of pier 1. Small areas of oyster growth were removed on each pier, with the underlying concrete proving to be in good condition.

Concrete cracking was found on piers 1, 2 and 3. Cracks were identified through the use of binoculars and by eye from the boat. Any cracks that were visible from the boat without the use of binoculars were noted as large (over 0.25mm), and cracks that were only visible by binoculars were noted as small (less than 0.25mm). See Appendix B Defect map for crack locations.

There were areas of spalling found on piers 1, 2, 3 and 4. However the likely cause was via impact damage/abrasions from boats. No rust staining was present at any of the spall locations.

Hammer tapping of the tidal zone provided no evidence to suggest any delamination of this area. However one area of rust staining was found on the south face of pier 2 at the MHWS level. Another area of rust staining was found on the north side of pier 2 at the western pier bracket connection. Rusting of old fixings were also found on the piers.

#### 2.1 Pier 1 north face

- Cracking. Approximately 8 cracks (1 large).
- 0.5m of crack injection required.
- Two areas of spalling (small).

#### 2.2 Pier 1 south face

- Cracking. Approximately 10 cracks (7 large).
- 6.0m of crack injection required.
- One area of spalling/impact damage to eastern corner.
- · Rust staining.





- Cracking. Approximately 3 cracks (1 large).
- 0.5m of crack injection required.

#### 2.4 Pier 2 south face

#### 2.5 Pier 3 north face

#### 2.6 Pier 3 south face

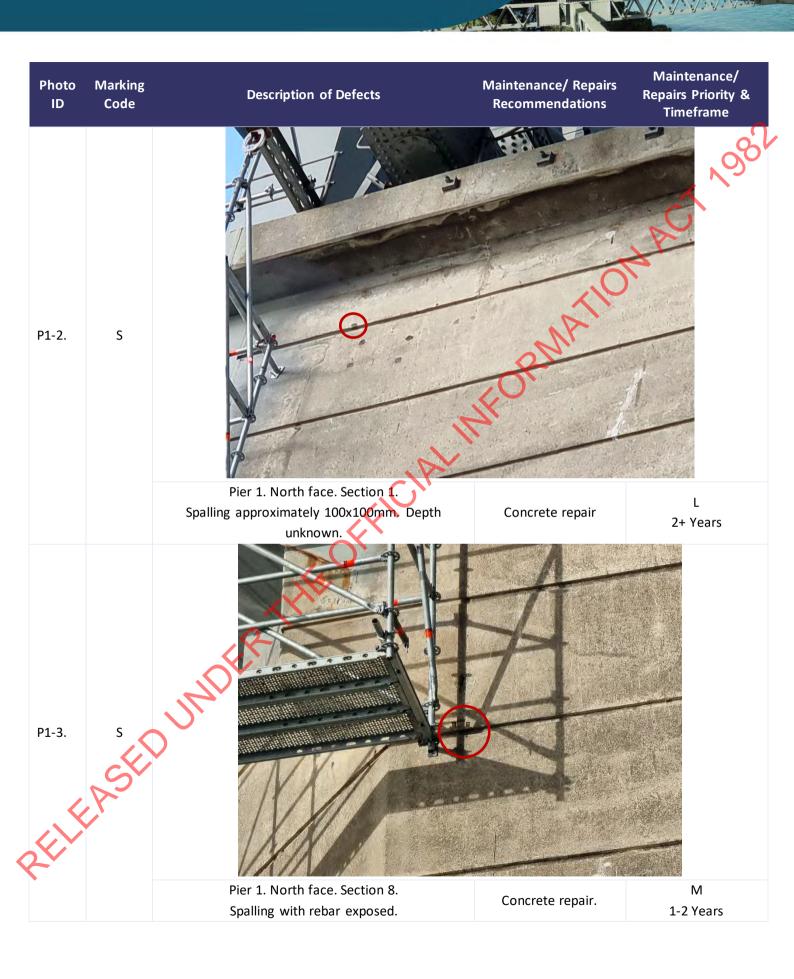
<ul> <li>Cracking. Approximately 3 cracks (1 large).</li> </ul>
0.5m of crack injection required.
One area of spalling/impact damage to eastern corner.
<ul> <li>O.5m of crack injection required.</li> <li>One area of spalling/impact damage to eastern corner.</li> <li>Rust staining.</li> </ul> 2.4 Pier 2 south face <ul> <li>Cracking. Approximately 8 cracks (4 large).</li> <li>3.0m of crack injection required.</li> <li>One Area of spalling/abrasion.</li> <li>Two old fixing holes.</li> <li>Rust staining.</li> </ul>
2.4 Pier 2 south face
Cracking. Approximately 8 cracks (4 large).
3.0m of crack injection required.
One Area of spalling/abrasion.
Two old fixing holes.
Rust staining.
<ul> <li>Two old fixing holes.</li> <li>Rust staining.</li> <li>2.5 Pier 3 north face</li> <li>Cracking. Approximately 4 cracks (all small).</li> <li>No crack injection required.</li> <li>Three areas of spalling.</li> <li>One old fixing hole.</li> <li>2.6 Pier 3 south face</li> </ul>
Cracking. Approximately 4 cracks (all small).
No crack injection required.
Three areas of spalling.
One old fixing hole.
2.6 Pier 3 south face
Cracking. Approximately 13 cracks (all small).
No crack injection required.
Two areas of spalling to recess edge.
Mark Priority No. Defects
R H - R
R L
S H -
S M 3
S L 15
Total 28
S L 15 Total 28



### 3 Defects

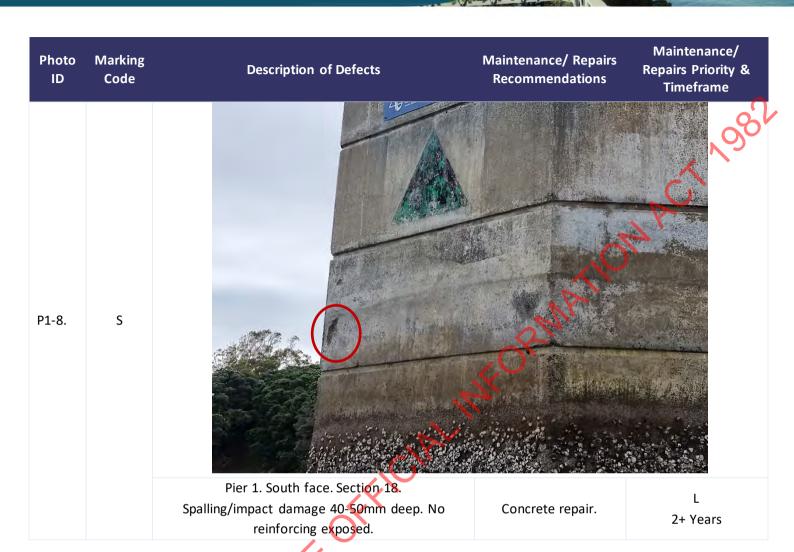
### 3.1 Pier 1













#### 3.2 Pier 2













### 3.3 Pier 3

	3.3 Fiel			
Photo ID	Marking Code	Description of Defects	Maintenance/ Repairs Recommendations	Maintenance/ Repairs Priority & Timeframe
P3-1.	N	Pier 3. North face. General view.	N/A	N/A
P3-2.	2 S			
2ELX		cracking (less than 6.25mm).	Monitor at next inspection	At next inspection.
		Abrasion/construction defect to recess edge.	Concrete repair.	2+ Years









### 3.4 Pier 4







Spallin

### 3.5 Pier 5

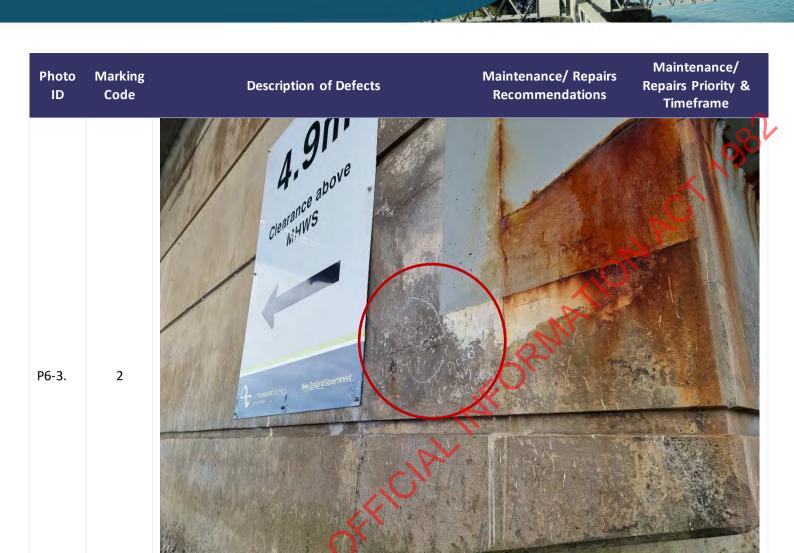
	3.5 FIE	1.3		
Photo ID	Marking Code	Description of Defects	Maintenance/ Repairs Recommendations	Maintenance/ Repairs Priority & Timeframe
P5-1.	N		4.9m	
		Pier 5. South face. General view.	N/A	N/A
P5-2.	N COLL		8.3m Citaring about	
		Pier 5. North face. General view.	N/A	N/A



Photo ID	Marking Code	Description of Defects	Maintenance/ Repairs Recommendations	Maintenance/ Repairs Priority & Timeframe
P5-3.	S	Pier 5. East corner.	Concrete repair.	M 1.2 Years
		Spalling/impact damage. Old repair failure.	·	1-2 Years

### 3.6 Pier 6





Pier 6. North west corner.

Hollow sounding delaminated concrete at circled

area.

RELEASEDUNDER

AGENCY

AGENCY

PART OF THE AUCKLAND MOTORWAY ALLIANCE

Monitor at next

inspection

At next inspection.

### Appendix A - Marking Code and Prioritisation of the Works

MATION ACT 1989 Defects are categorised by marking code, as used in NZTA S6 Bridges and other significant highway structures inspection policy:

- 0 = Not inspected
- 1 = Satisfactory
- 2 = Monitor next inspection
- R = Routine maintenance
- **S** = Structural maintenance
- N = Not applicable

The following timeframes are used for prioritising maintenance works:

- High must be done as soon as possible, within 12 months
- **Medium** preferable to do as soon as possible, within 1-2 years
- Low no work required within the next 2 years OFFICIAL ST

PELLERSED Appendix B - Piers 1-3 Defect Map

